

CLASS NOTES

LECTURE # 2 & 3

(pg. 1 → pg. 11)

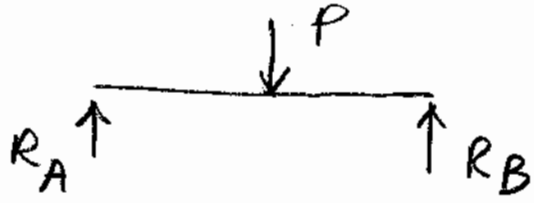
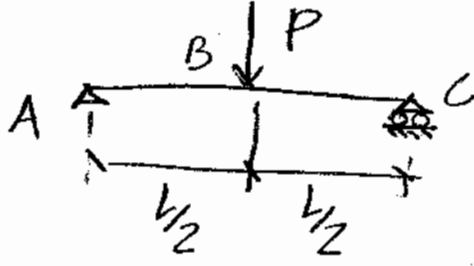
SIMPLY SUPPORTED BEAMS

BRACKETS, BRACINGS

(09/08/2005)

EXAMPLE: SIMPLY SUPPORTED BEAM LOADED W/ CONC. LOAD P, FIND REACTIONS.

DRAW FREE BODY DIAGRAM F.B.D.



$$\sum F_x = 0$$

$$\sum F_y = 0$$

$\sum M = 0 \rightarrow$  TAKE MOMENT  $\sum$  ABOUT ONE OF SUPPORTS  $\rightarrow$  ELIMINATE ONE UNKNOWN REACTION.

BY OBSERVATION, SINCE SYM. LOADING  $\rightarrow$  REACTIONS WOULD HAVE TO BE EQUAL.  $R_A = R_B$  &  $= P/2$   
AS  $\sum F_y = 0$ .

BRUTE FORCE METHOD:

$$\sum F_x = 0 \text{ AS ONE END SITS ON ROLLER } \Rightarrow C_x = 0$$

$$\text{AS } C_x = 0 \Rightarrow A_x = 0 \text{ SINCE } C_x + A_x = 0$$

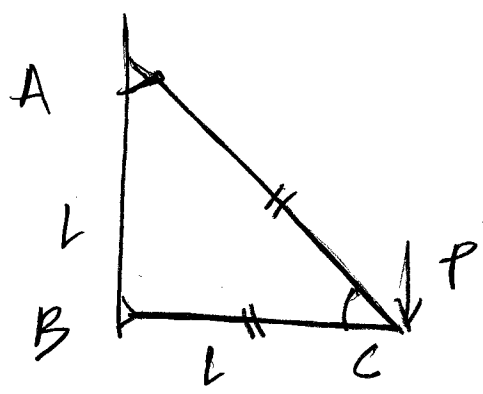
$$\sum F_y = 0 \Rightarrow R_A + R_C - P = 0 \Rightarrow R_A + R_C = P \text{ EQ (1)}$$

$\sum M_A = 0$  RIGHT HAND RULE (SCREW)

$$\underbrace{-P}_{\text{FORCE}} \underbrace{\frac{L}{2}}_{\text{DISTANCE}} + R_C L + R_A(0) = 0 \Rightarrow R_C L = P \frac{L}{2} \therefore R_C = \frac{P}{2}$$

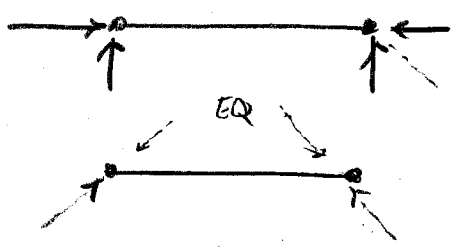
$$\text{PLUG IN EQ (1): } R_A = P - R_C = P - \frac{P}{2} = \frac{P}{2}$$

# SOLVE A BRACKET :

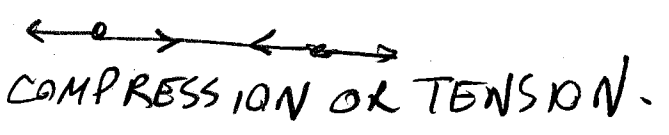


TWO FORCE MEMBER  
3 FORCE MEMBER.

2 FORCE MEMBER IS;

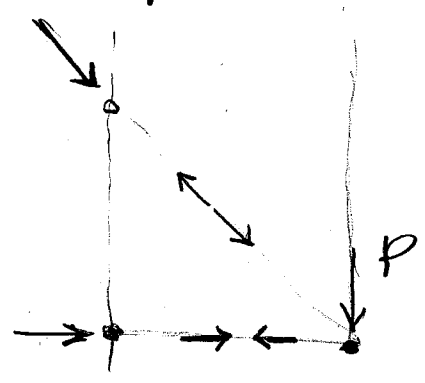


TRUSS MEMBER W/ PINS.



BOTH MEMBERS AC & BC ARE TWO FORCE MEMBERS

EXTERNAL REACTIONS @ A & B WOULD HAVE TO MEET P @ A POINT.

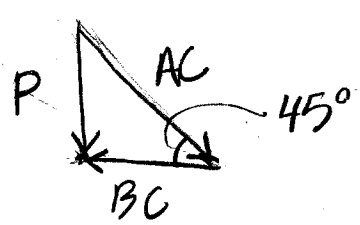


↳ THAT POINT IS C

⇒ BC IS IN COMPRESSION.

AC IN TENSION.

GRAPHIC POLYGON.



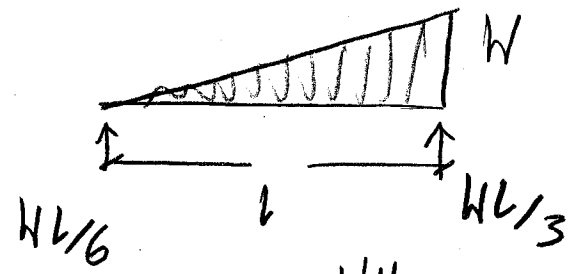
$$\sin 45^\circ = \frac{P}{AC}$$

$$AC = \frac{P}{\sin 45^\circ} = \frac{P}{\frac{1}{\sqrt{2}}}$$

$$\sin 45^\circ = \frac{\sqrt{2}}{2}$$

$$AC = \frac{2P}{\sqrt{2}} = \frac{2P\sqrt{2}}{2} = P\sqrt{2}$$

$$\boxed{BC = P}$$



$$\sum F_x = 0 \checkmark$$

$$\sum F_y = 0 \Rightarrow R_A + R_B = Wl/2$$

$$\sum M_A = 0 \Rightarrow -Wl/2 \cdot 2l/3 + R_B l = 0$$

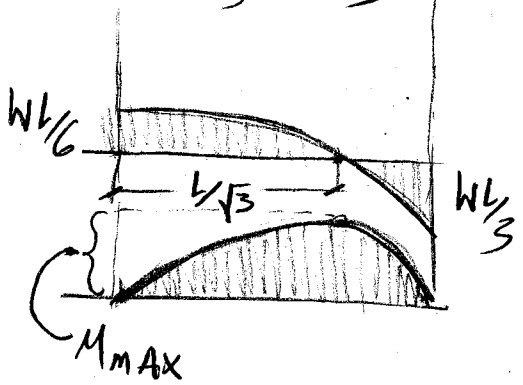
$$R_B = Wl/3$$

$$\Rightarrow R_A = Wl/2 - Wl/3 = Wl/6$$

$$V(x) = R_A - Wx/2 \quad \text{WHEN } V(x) = 0$$

$$\Rightarrow V(x) = Wl/6 - Wx/2 = 0$$

$$\Rightarrow x/2 = l/6 \Rightarrow x^2 = l^2/6 \Rightarrow x = l/\sqrt{3}$$

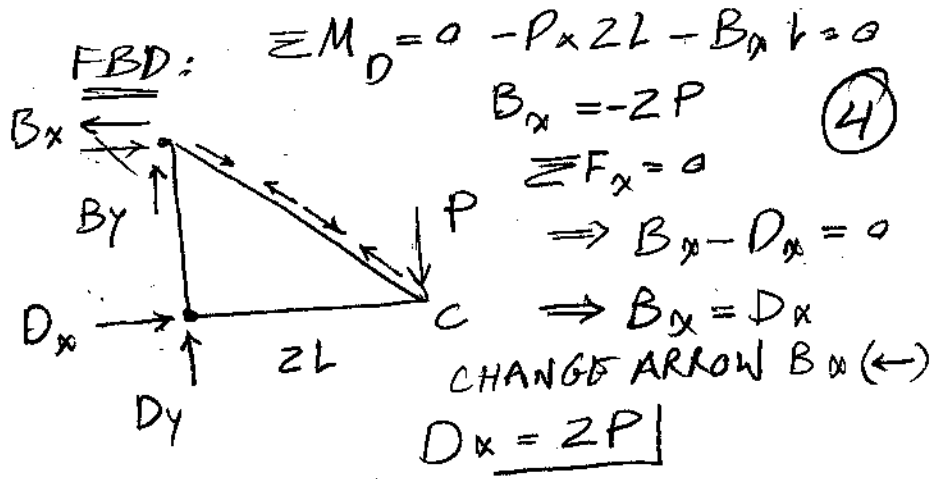
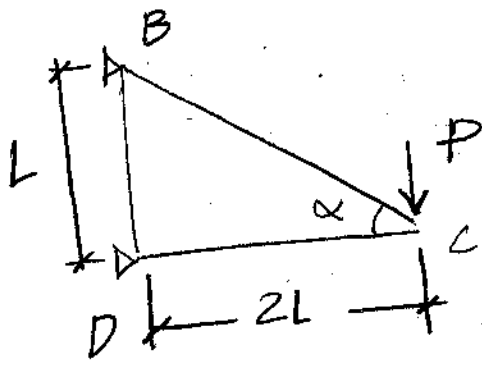


$$M(x) = R_A x - Wl/6 x \cdot x/2 \cdot 1/3 x = Wl/6 x - \frac{Wx^3}{6L}$$

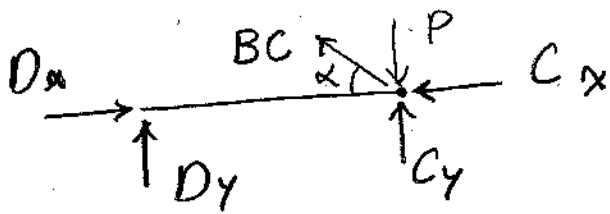
$$M_{\max} = M\left(\frac{l}{\sqrt{3}}\right) = Wl/6 \cdot \frac{l}{\sqrt{3}} - \frac{W}{6L} \cdot \frac{l}{\sqrt{3}} \cdot \frac{l}{\sqrt{3}} \cdot \frac{l}{\sqrt{3}}$$

$$= \frac{Wl^2}{6\sqrt{3}} - \frac{W}{\sqrt{3}} \cdot \frac{l^2}{18}$$

$$= \frac{Wl^2}{6\sqrt{3}} - \frac{Wl^2}{18\sqrt{3}} = \frac{3Wl^2 - Wl^2}{18\sqrt{3}} = \frac{Wl^2}{9\sqrt{3}}$$



TAKE ELEMENT DC FBD



$$\alpha = \tan^{-1}\left(\frac{L}{2L}\right) = \tan^{-1} 0.5 = 26.56^\circ$$

AT JOINT C:  $\sum F_y = 0$

$$-P + BC \sin \alpha + C_y = 0 \quad (1)$$

$$\sum F_x = 0 \Rightarrow -BC \cos \alpha + D_x = 0$$

$$\Rightarrow -BC \cos 26.56 + D_x = 0 \text{ BUT } D_x = 2P (\rightarrow)$$

$$D_x = BC(0.89) \Rightarrow 2P = BC(0.89) \Rightarrow BC = 2.23P$$

$$(1) -P + BC \sin 26.56 + C_y = 0$$

$$\Rightarrow -P + 2.23(0.4471)P + C_y = 0 \Rightarrow C_y = P$$

JOINT B:  $\sum F_y = 0 \Rightarrow -2.23P \cos \beta + B_y = 0$

$$\Rightarrow B_y = 2.23P \cos \beta \text{ BUT } \beta = 180 - 90 - \alpha$$

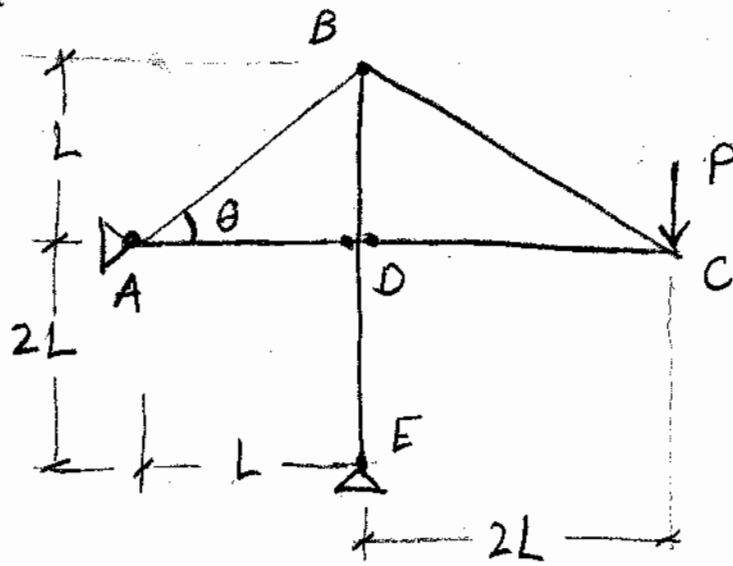
$$\beta = 90 - \alpha = 90 - 26.56 = 63.44^\circ$$

$$\Rightarrow B_y = 2.23P (\cos 63.44) = P$$

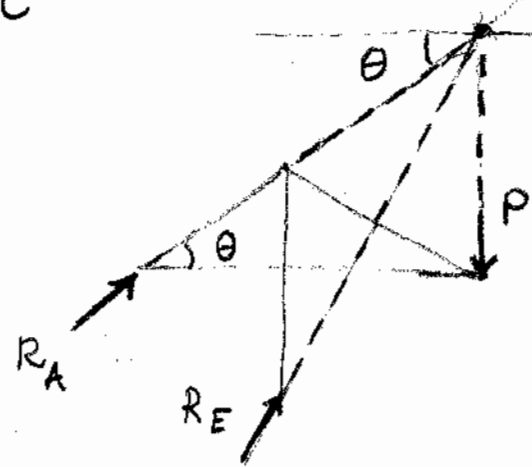
JOINT D:  $\sum F_x = 0$   
 $\sum F_y = 0 = D_y$  QED

PROBLEM:

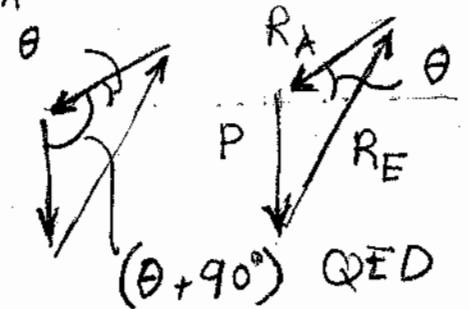
(5)



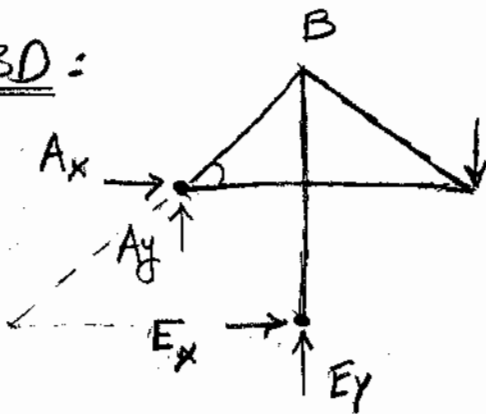
GRAPHICAL METHOD:



$R_E$  COMPRESSION  
 $A_A$  TENSION  $\Rightarrow$  REVERSE



FBD:



$\theta = \tan^{-1}(L/2) = 45^\circ$

$\sum F_y = 0 \Rightarrow A_y + E_y - P = 0$

$\Rightarrow A_y + E_y = P \quad (1)$

$\sum F_x = 0 \Rightarrow A_x + E_x = 0$

$\sum M_E = 0 \quad A_x = -E_x \quad (2)$

$-P \cdot 2L + A_y \cdot L + A_x \cdot 2L = 0 \quad (3)$

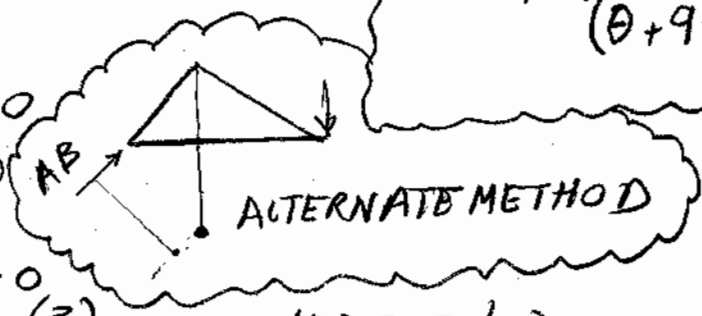
$\sum M_A = 0$

$-P \cdot 3L + E_y \cdot L + E_x \cdot 2L = 0 \quad (4)$

PLUG (1) INTO (4)  $\Rightarrow E_y = P - A_y$

$-A_y \cdot L + 2E_x \cdot L = 2PL \quad (5)$

$\Rightarrow A_y - 2E_x = 2P \quad (6)$



PLUG (2) INTO (3)

$+2PL + A_y \cdot L - E_x = 0 \quad (6)$

INTO (4)  $\Rightarrow -3PL + PL - A_y \cdot L + 2E_x \cdot L = 0$

$$-PL(2) + A_y L + A_x 2L = 0 \quad (\text{EQ \#3})$$

$$-3PL + E_y L + E_x 2L = 0 \quad (\text{EQ \#4})$$

$$(3) A_y L + A_x 2L = 2PL$$

$$(4) E_y L + E_x 2L = 3PL$$

$$(6) A_y L - 2E_x L = 2PL$$

$$\Rightarrow E_y L + A_y L = 5PL$$

$$\Rightarrow E_y + A_y = 5P$$

$$E_y + A_y = P$$

$$\begin{cases} A_y + E_y = P \\ A_y + 2A_x = P \end{cases} \Rightarrow E_y - 2A_x = 0 \Rightarrow E_y = 2A_x$$

$$E_y + 2E_x = 3P \quad \text{or} \quad A_x = -E_x$$

$$\begin{cases} E_y + 2E_x = 3P \\ A_y + 2A_x = P \end{cases}$$

$$E_y - 2A_x = 3P$$

$$A_x = -E_x$$

$$E_y = 2A_x$$

$$A_x = E_y/2$$

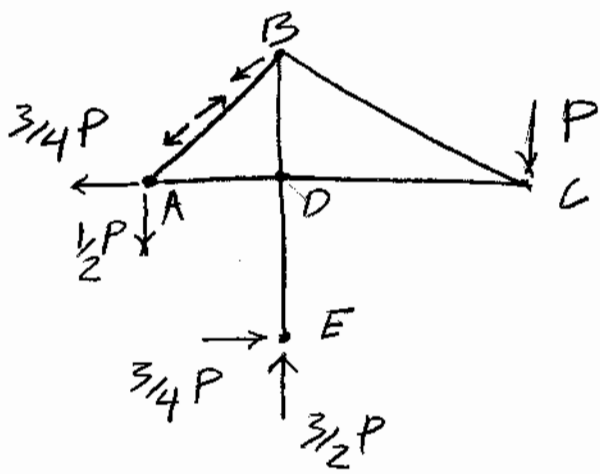
$$\Rightarrow E_y/2 = E_x$$

$$\Rightarrow E_y = 2E_x$$

$$E_y + 2E_x = 3P \Rightarrow 2E_x + 2E_x = 3P \Rightarrow 4E_x = 3P$$

$$\Rightarrow E_x = 3/4 P \quad A_x = -3/4 P$$

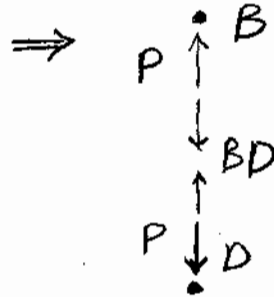
$$E_y = 2E_x = 2(3/4 P) = 3/2 P \quad A_y = P - 3/2 P = -1/2 P$$



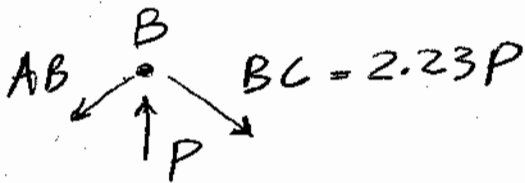
FIND FORCE IN MEMBER DE. (7)

FROM PREVIOUS PROBLEM.

$$B_y = P \uparrow \quad D_y = 0$$

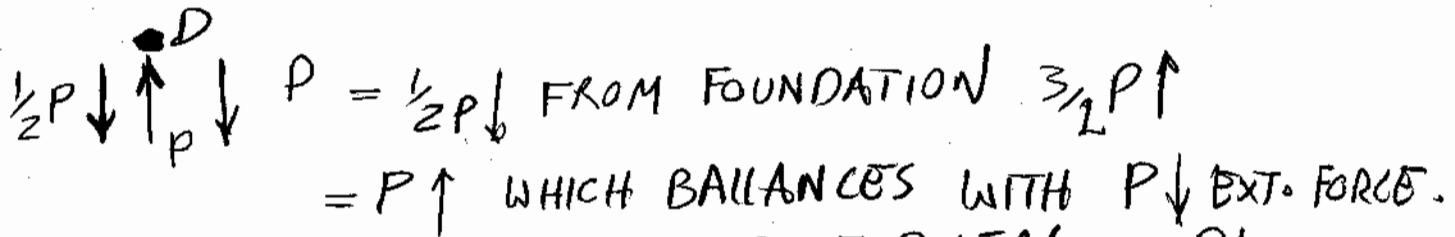


TAKE JOINT B:



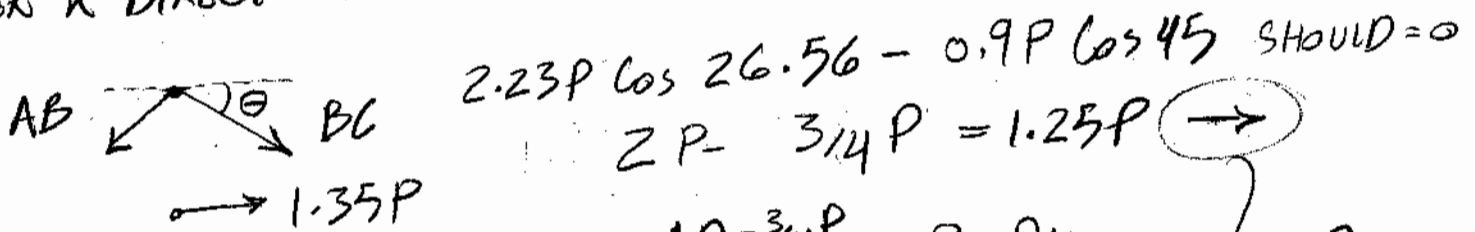
$$AB = \sqrt{A_x^2 + A_y^2} = \sqrt{(-3/4 P)^2 + (1/2 P)^2} = \sqrt{0.5625 + 0.25} = 0.9 P$$

$$A_y = -1/2 P \rightarrow B_{0y} = -2.23 P \times \cos 63.44 = -P$$



@ JOINT D. THEREFORE RIGHT SIDE SHEAR =  $P \downarrow$   
LEFT SIDE SHEAR =  $-1/2 P$  OR  $1/2 P \downarrow$

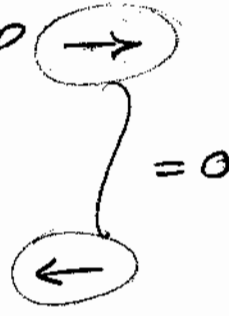
FOR X DIRECTION TAKE JOINT B.



JOINT D: X DIR  $D_x = 2P$

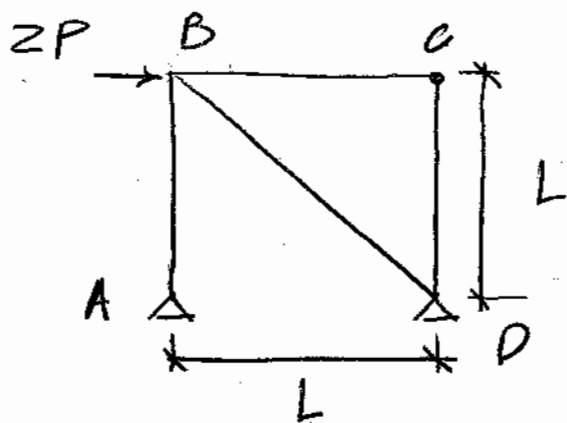
$$AD = 3/4 P \quad 2P = D_x$$

$$2P - 0.75P = 1.25P$$



⇒ LOOKING AT COLUMN EB THERE IS A MOMENT AT JOINT D  $M = 1.25 P \times L$

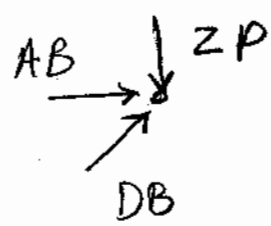
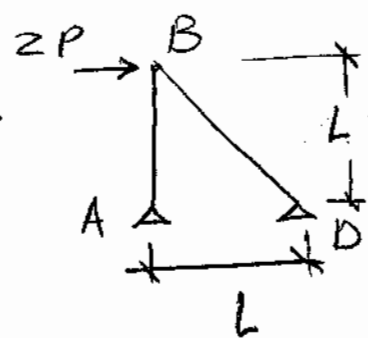
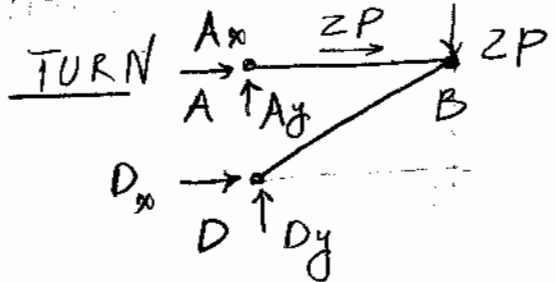




BRACING:

NOTE AS MEMBERS BC & CD @ JOINT C HAVE NO FORCE  $\Rightarrow$  THEY ARE ZERO FORCE MEMBERS.

• GET RID OF THEM & WE HAVE A BRACKET.



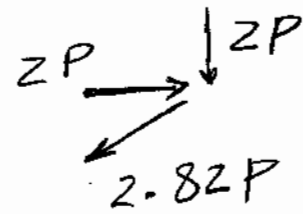
DB IS A TWO FORCE MEMBER  $\Rightarrow$  @ JOINT B

$DB \sin \theta = -ZP \Rightarrow DB = -ZP / \sin \theta = -ZP / 0.7071$

$DB \cos \theta + AB = 0 \Rightarrow DB = -2.8284$

$DB (0.7071) = -AB \Rightarrow -2.8284 (0.7071) = -AB$

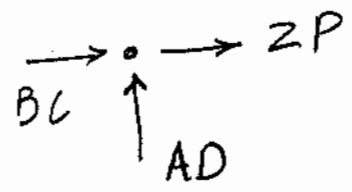
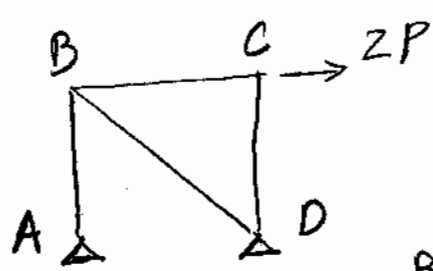
$AB = ZP$



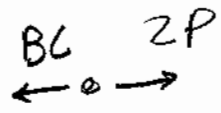
REVERSE ARROW FOR DB.

IF LOAD IS MOVED TO JOINT C:

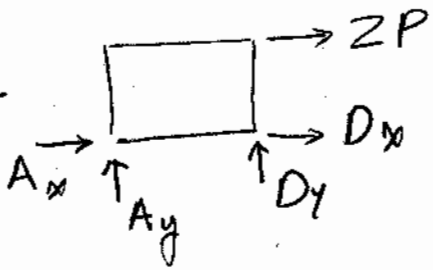
AT JOINT C:  $\sum F_x = 0$



$BC + ZP = 0 \Rightarrow BC = -ZP$



GLOBAL STRUCTURE



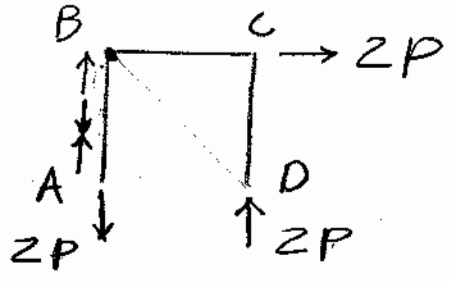
$\sum F_x = 0$   
 $\sum F_y = 0$   
 $\sum M_A = 0$

$$\sum F_x = 0 \Rightarrow A_x + D_x + 2P = 0$$

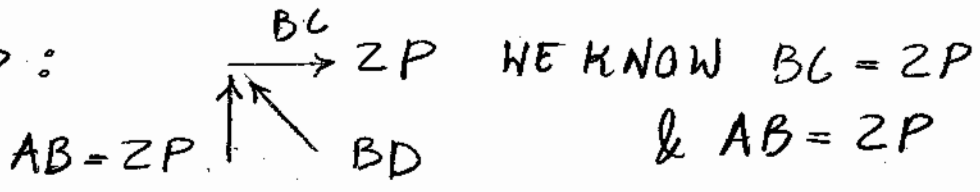
$$\sum F_y = 0 \Rightarrow A_y + D_y = 0 \Rightarrow A_y = -D_y$$

$$\sum M_A = 0 \Rightarrow -2PL + D_yL = 0 \Rightarrow D_y = \underline{2P} \uparrow$$

$$\rightarrow A_y = -2P \downarrow$$



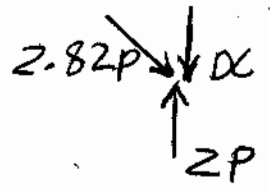
AT JOINT B:



WE KNOW  $BC = 2P$   
&  $AB = 2P$   
 $\Rightarrow$  BD WOULD HAVE TO PUT JOINT B IN EQUILIBRIUM.

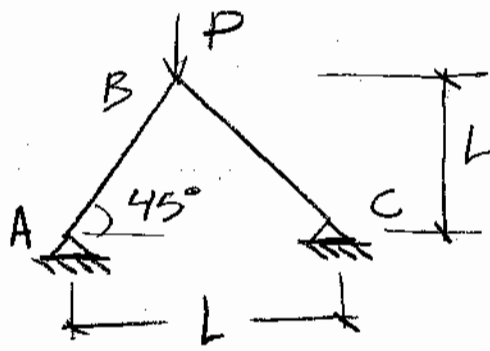
$$BD = \sqrt{(2P)^2 + (2P)^2} = \sqrt{8P^2} = 2.8284P$$

Q AT JOINT D

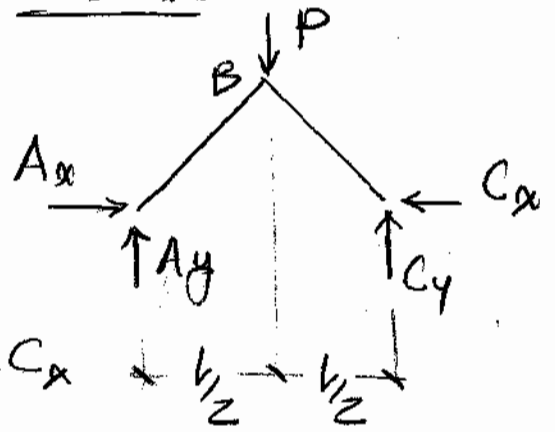


WE SEE: MEMBER DC TAKES NO LOAD. DC = 0

# ARCHES:



## F.B.D.:



(10)

$$\sum F_x = 0 \Rightarrow A_x - C_x = 0 \Rightarrow A_x = C_x$$

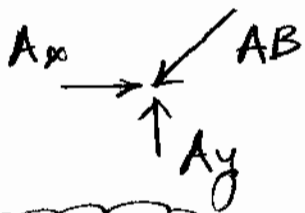
$$\sum F_y = 0 \Rightarrow A_y + C_y - P = 0$$

$$\sum M_A = 0 \Rightarrow -P(L/2) + C_y L + A_x(0) + A_y(0) + C_x(0) = 0$$

$$\Rightarrow C_y = \frac{P}{2}$$

$$\sum F_y = 0 \Rightarrow A_y = P - C_y = P - \frac{P}{2} = \frac{P}{2}$$

MEMBER AB IS A TWO FORCE MEMBER  $\Rightarrow$  AT JOINT A:



$$\sum F_x = 0$$

$$-AB \cos \theta + A_x = 0 \Rightarrow A_x = 0.7071 AB$$

$$\sum F_y = 0 \Rightarrow -AB \sin \theta + A_y = 0$$

$$\Rightarrow A_y = 0.7071 AB \text{ BUT } A_y = \frac{P}{2}$$

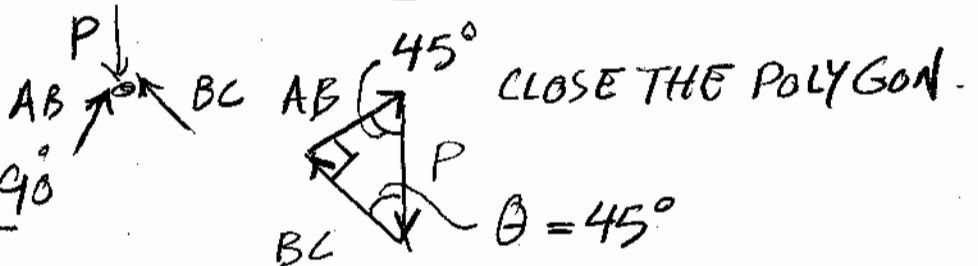
$$\Rightarrow AB = \frac{A_y}{0.7071} = \frac{P/2}{0.7071} = \frac{P}{2} \times \frac{2}{\sqrt{2}} = \frac{P\sqrt{2}}{2}$$

$$AB_x = \frac{P\sqrt{2}}{2} \times \frac{\sqrt{2}}{2} = \frac{P(2)}{4} = \frac{P}{2}$$

$$AB_x = A_x ; C_x = \frac{P}{2} \text{ AS WELL. QED}$$

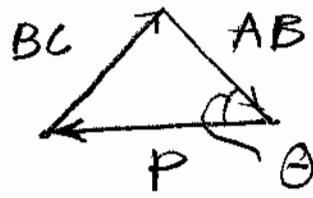
## GRAPHIC METHOD:

$$\frac{\sin 45^\circ}{AB} = \frac{\sin 45^\circ}{BC} = \frac{\sin 90^\circ}{P}$$

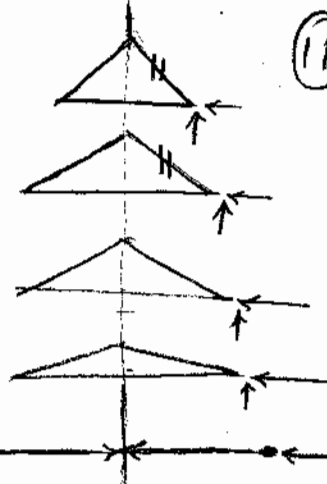


ROTATE GRAPHIC

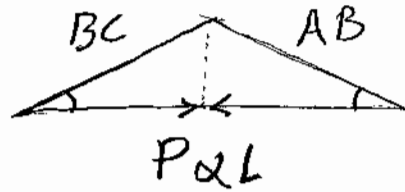
THINK OF P AS THE SPAN OF ARCH.



(11)



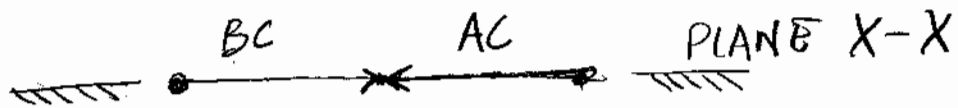
AS WE DECREASE ANGLE theta SPAN INCREASES



$P \propto L$

LOAD P IS PROPORTIONAL TO SPAN L. NEXT THINK OF BC & AB AS PROJECTION OF LOAD OR THRUST AT SUPPORT OF ARCH.

AS WE DECREASE ANGLE MORE & MORE WE REACH A FLAT LINE



THIS SHOULD BE THE MAXIMUM FORCE COMPRESSIVE WHICH THE MEMBERS SHOULD BE DESIGNED FOR & AT THIS POINT THE ARCH BECOMES A BEAM. & THE SHEAR FORCE BECOMES A FACTOR IN DESIGN. + BENDING MOMENT.

AS WE GO BELOW THE PLANE X-X, THE ARCH BECOMES A CABLE & THE REVERSE IS TRUE & VERTICAL & HORIZONTAL FORCES AT THE SUPPORT FORM INTO COMPRESSIVE AXIAL LOAD.  $\frac{P}{2}$  @ 45°

